

SEAOC Seismic Design Manual (1997 UBC Version)  
Errata No. 2 for Volume II

**Page 74, Design Example 1.**

Add the following comment at the bottom of the page:

It is not clear in the code whether the  $E_m$  force needs to be applied to the tiedowns for shear walls. The SEAOC Seismology Committee has recently formed the opinion that the  $E_m$  force need only apply to compression forces and not tensile forces in the member.

**Page 152, Design Example 2.**

In Part 19, change first paragraph to:

Sections 2304.3 of the 1997 UBC appears to have changed the requirements for corrosion-resistant fasteners in treated sill plates. Although it does not appear to be the intent of the provision, a literal interpretation of the section would require hot-dipped zinc coated galvanized nails and anchor bolts. During the re-write of the wood chapter from the 1994 UBC to the 1997 UBC, the exception (§2311.1 in the 1994 UBC) unexplainably was omitted. The code change was proposed by the wood industry, and §2304.3 is from a report in the wood handbook by the Forest Products Lab where fasteners were found to react with the preservative treatment when "...in the presence of moisture..." This can create a construction problem because hot-dipped zinc coated nails have to be hand driven, requiring the framer to put down his nail gun and change nailing procedures. The wording in the 2000 IBC is the same as the 1997 UBC. On May 31, 2001 in a letter from ICBO to the California Building Industry Association, ICBO has now classified this as an "inadvertent omission" and has added an errata as follows:

"Exception: When approved, fasteners used in contact with preservative-treated wood products when not below grade or exposed to weather."

**Page 223, Design Example 4.**

In the equation given on line 10, change " $h_x = 0$ " to " $h_x = 16'$ ".

**Page 232, Design Example 4.**

Line 13, change to read:

$$1.1 (1.2D + 0.27D + 1.0 E_h) = 1.61D + 1.1 E_h$$

Line 15, change equation to read:

$$1.1 (0.9D - 0.27D + 1.0E_h) = 0.69D - 1.1E_n$$

**Page 274, Design Example 5.**

At the end of Part 6d, add the following paragraph:

**Comment:** Section 1633.2.6 disallows the use of the one-third stress increase in determining strength to resist force  $E_m$  when Allowable Stress Design is used. Use of this approach has been shown above. However, if the duration of load increase permitted by §1633.2.6 is used, an additional 1.33 increase in strength to resist  $E_m$  is obtained. This is from Table 2.3.2, “Load Duration Factors,  $C_D$ ” of the code. Use of the “load duration” factor approach will result in a smaller increase in GLB size than that shown above.

**Page 285, Design Example 5.**

Line 19, change “0.0025” to “0.025.”